

FINAL EXAMINATION (ONLINE) SEMESTER II SESSION 2019/2020

COURSE NAME :

DESIGN OF WATER SUPPLY

COURSE CODE

: BFA 40203

PROGRAMME

BFF

EXAMINATION DATE:

JULY 2020

DURATION

6 HOURS

INSTRUCTION

ANSWER FIVE (5) QUESTIONS ONLY

QUESTION Q1 IS COMPULSORY

THIS QUESTION PAPER CONSISTS OF SIX (6) PAGES UKA

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Prepare a design of a water treatment system that is suitable for a proposed development area 01 which consist of 500 units double-story terrace and 30 units bungalow houses. (20 marks) Formulate general equation for water demand. Q2 (a) (4 marks) A city recorded populations of 220,000 and 250,000 in April 1990 and April 2000 (b) respectively. Estimate the population in January 2025. (6 marks) Table 1 Q2 (c) represents the lowest seven consecutive day average discharge from (c) 2000 to 2018. The river supply is intended for abstraction to meet an average demand of 7 m3/s of a community. Tabulate the flows in order of severity using serial number M with values from (i) 1 to n. (2 marks) Plot the flows against their probability. (ii) (4 marks) Determine the minimum flow for a 10-year return period. (iii) (4 marks) Propose a suitable water treatment system in a limited space of treatment plant's area Q3 (a) (4 marks)

- Estimate the detention times required in completely mixed and plug-flow reactors for (b) an 80 % reduction of reactant concentration. Given;
 - Influent reactant concentration, C₀ is 250 mg/L (i)
 - Effluent reactant concentration, Ct is 50 mg/L (ii)
 - Rate constant for a chemical coagulation reaction, K is 75 per day. (iii)

(6 marks)

- Design a flocculation basin by determining the basin volume, tank dimensions and (c) required input power using the following data;
 - (i) Flocculation basin is 2 unit
 - Design flowrate is 10 m³/min (ii)
 - Detention time is 20 min (iii)
 - (iv) Water depth is 4.5 m
 - Compartment is 2 unit (v)
 - Velocity gradient, G in each compartment are 40 stand 35 UKA (vi)
 - Dynamic viscosity at 24°C is 0.000911 Pa.s (vii)

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(viii) Efficiency of transfer of motor power to water power is 80 %

(10 marks)

Q4 (a) Sketch FOUR (4) zones in the sedimentation basin

(3 marks)

(b) Differentiate with an example between Type I Sedimentation and Type II Sedimentation and give an example of where they are applied in water treatment

(7 marks)

- (c) Design the horizontal-flow rectangular sedimentation tank(s) for Parit Raja Water Treatment Plant using the maximum day design flow of 0.5 m³/s by using the following:
 - (i) The surface overloading rate is $40 \text{ m}^3/\text{day.m}^2$.
 - (ii) 4 tanks is prepared and a width of each tank is 4 m.
 - (iii) Minimum length to width ratio (L: W) is 6:1
 - (iv) Depth of the tank is 2 m
 - (v) Minimum length to depth ratio (L: D) is 15:1
 - (vi) Assess on the turbulence and backmixing if the water temperature is 15°C (kinematic viscosity is 1.14 x 10 -6 m²/s).
 - (vii) Minimum horizontal flow ranges between 0.005 0.018 m/s
 - (viii) Reynoulds number < 20,000

(10 marks)

- Q5 (a) Explain the basic requirement of pressure and capacity in water distribution system. (4 marks)
 - (b) The water level in the service tower (elevated tank) supplying the residential area is 500 m. The elevation of the supply line at the residence is 400 m. Assume the head loss is 1 m. Calculate
 - (i) Static head
 - (ii) Static pressure
 - (iii) Dynamic head
 - (iv) Dynamic pressure (actual pressure)

(6 marks)

(c) A service reservoir is to be designed for a water supply serving 250.000 people with an average demand of 225 L/day/capita and a fire flow of 37000 L/min. Given the peak demand can be taken as 25% of the maximum daily demand, maximum daily demand factor is 1.8 times the average demand and Fire demand should be maintained at least for 3 hours.

Calculate:

- (i) Maximum daily flowrate
- (ii) Volume at peak demand
- (iii) Volume to supply the fire demand



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- (iv) Emergency storage
- (v) Total volume of the service reservoir

(10 marks)

Q6 (a) Explain Hardy Cross method for pipe network analysis

(4 marks)

(b) Using Hardy Cross Method, calculate the flow rate for each pipe as shown in **Figure Q6** (b).

(6 marks)

(c) A distribution network is shown in Figure **Figure Q6(c)**. Calculate the velocity for each distribution pipe and determine the residual pressure at A and B by using head loss due to conveying water from reservoir R to point A and from Point A to B are 6.04 m and 6.03 m respectively.

(10 marks)

-END OF QUESTIONS-



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2000 34.3 2001 29.3 2002 35.7 2003 35.0 2004 27.0 2005 35.0 2006 36.9 2007 50.6 2008 35.3 2009 59.4 2010 26.3 2011 30.1 2012 29.4 2013 29.7 2014 90.3 2015 30.4 49.6	Year	River Discharge, m ³ /s
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		90.3
	2015	30.4
2010	2016	49.6
2017 36.6		36.6
2018 59.1	2018	59.1

TABLE Q2(c): The lowest seven consecutive day river discharge



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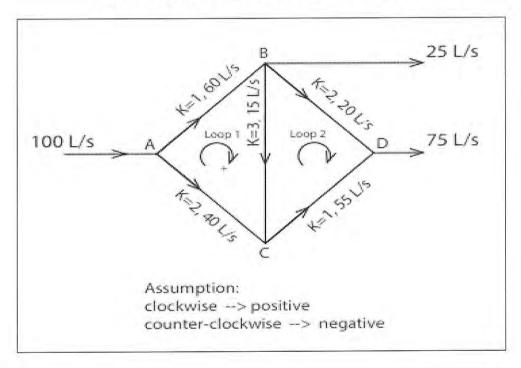
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FIGURE Q6 (B): Proposed flow rate for a pipe distribution



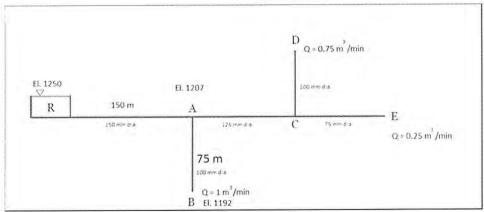


FIGURE Q6 (c): A distribution network