



## **UNIVERSITI TUN HUSSEIN ONN MALAYSIA**

### **PEPERIKSAAN AKHIR SEMESTER II SESI 2008/2009**

NAMA MATA PELAJARAN : REKABENTUK STRUKTUR KONKRIT  
KOD MATA PELAJARAN : BFC 3113 / BKA 3123  
KURSUS : BFF / BKA  
TARIKH PEPERIKSAAN : APRIL 2009  
JANGKA MASA : 3 JAM  
ARAHAN : JAWAB EMPAT (4) SOALAN SAHAJA

SEMUA KIRAAN HENDAKLAH  
BERPANDUKAN KEPADA  
PIAWAIAN:  
BS 8110: PART 1: 1997  
BS 8110: PART 2: 1985  
BS 8110: PART 3: 1985  
BS 6399: PART 1: 1984

- S1** Rajah S1(a) menunjukkan sebahagian lantai bawah bagi rumah dua tingkat dengan rasuk dan papak dituang secara monolitik. Dengan menggunakan data berikut:

Tebal papak	=	125 mm
Kemasan	=	1.0 kN/m <sup>2</sup>
Berat dinding bata : tinggi 3 m	=	2.6 kN/m <sup>2</sup>
Kekuatan ciri konkrit	=	30 N/mm <sup>2</sup>
Penutup konkrit	=	25 mm
Beban kenaan ciri	=	2.5 kN/m <sup>2</sup>
Tetulang utama, $\phi_{bar}$ : 20 mm	=	Keluli alah tinggi
Tetulang ricih, $\phi_{lmsk}$ : 8 mm	=	Keluli alah sederhana
Saiz rasuk, b <sub>w</sub> x h	=	250 mm x 500 mm

- (a) Buktikan beban rekabentuk muktamad bagi rasuk 2/A-C adalah sama seperti yang ditunjukkan dalam Rajah S1(b). (4 markah)
- (b) Rekabentukkan rasuk 2/A-C sekiranya daya ricih dan momen lentur maksimum yang diperoleh dari analisis adalah 281.96 kN dan 216.93 kNm. (13 markah)
- (c) Semak had kebolehkhidmatan rasuk untuk lenturan dan keretakan sahaja. (5 markah)
- (d) Berikan komen jika rasuk T atau L direkabentuk sebagai rasuk segiempat. (3 markah)

- S2** (a) Definisi papak. (3 markah)
- (b) Berbantukan lakaran, senaraikan **TIGA (3)** jenis papak. (3 markah)
- (c) Sebuah papak selanjar dengan ketebalan 150 mm seperti yang ditunjukkan dalam Rajah S2. Data-data rekabentuk adalah seperti berikut.

Ketumpatan konkrit	= 24 kN/m <sup>3</sup>
Kemasan	= 0.5 kN/m <sup>2</sup>
Beban hidup ciri	= 2.5 kN/m <sup>2</sup>
Gred konkrit	= 35
Gred keluli	= 460
Penutup konkrit	= 20 mm
Diameter bar (anggapan)	= 16 mm

Untuk papak Panel 1,

- (i) Rekabentukkan semua tetulang. Tetulang puntiran diabaikan. (5 markah)

- (ii) Lakukan semakan rincih. (3 markah)
- (iii) Semak lenturan dan keretakan. (3 markah)
- (iv) Lukiskan perincian. (3 markah)
- (d) Papak padu yang disokong di keempat-empat sisinya memindahkan beban melalui rasuk. Namun begitu, bagi kes-kes tertentu seperti papak rata, beban dipindahkan dari papak terus kepada tiang. Bincangkan kriteria yang kritikal bagi papak rata. (5 markah)

S3 Rajah S3(a) menunjukkan pelan lantai sebuah bangunan dua tingkat yang dirembat. Saiz semua tiang ialah  $250 \times 300$  mm dan saiz rasuk utama dan kedua masing-masing ialah  $250 \times 500$  mm dan  $250 \times 400$  mm. Rajah 3(b) menunjukkan nilai momen lentur tiang B/3 pada paksi X-X yang dianalisa menggunakan analisis kerangka separa. Data rekabentuk diberi seperti berikut:

(i) Bebanan (semua aras termasuk bumbung)

Rasuk utama:	
Beban mati ciri (termasuk beban kendiri)	= $16 \text{ kN/m}$
Beban kenaan ciri	= $20 \text{ kN/m}$

Rasuk kedua:	
Beban mati ciri (termasuk beban kendiri)	= $12 \text{ kN/m}$
Beban kenaan ciri	= $15 \text{ kN/m}$

(ii) Sifat bahan:

Kekuatan ciri konkrit	= $35 \text{ N/mm}^2$
Kekuatan ciri tetulang utama	= $460 \text{ N/mm}^2$
Kekuatan ciri perangkai	= $250 \text{ N/mm}^2$
Penutup konkrit	= $30 \text{ mm}$

Untuk tiang tingkat bawah B/3,

- (a) Klasifikasikan jenis tiang. (4 markah)
- (b) Kirakan daya paksi pada tiang. (6 markah)
- (c) Rekabentukkan semua tetulang pada tiang dengan menganggap momen lentur pada paksi Y-Y adalah terlalu kecil. (10 markah)

- (d) Pada pendapat anda, apakah perubahan rekabentuk yang perlu dibuat jika tiang bulat digunakan selain tiang segi empat tepat.  
(5 markah)

- S4** Rajah S4 menunjukkan tangga yang disokong mudah oleh rasuk 2/A-B dan B/1-2. Pelantar pula disokong oleh rasuk A/1-2 dan 1/A-B. Beban kenaan ciri tangga dan kemasan adalah  $5.0 \text{ kN/m}^2$  dan  $0.75 \text{ kN/m}^2$ . Bahar binaan terdiri daripada konkrit gred 30 dan menggunakan keluli alah tinggi untuk semua tetulang. Data rekabentuk diberi seperti berikut:

Jejak, G	=	250 mm
Penaikan, R	=	170 mm
Tebal cekak, h	=	150 mm
Tebal pelantar	=	175 mm
Penutup konkrit	=	25 mm

- (a) Rekabentukkan semua tetulang bagi tangga dan pelantar.  
(11 markah)
- (b) Lakukan semakan rincih, pesongan dan keretakan.  
(6 markah)
- (c) Lukiskan perincian tangga.  
(4 markah)
- (d) Sekiranya berlaku kegagalan bagi semakan pesongan, apakah penyelesaian yang perlu anda buat.  
(4 markah)

- S5** (a) Senaraikan **TIGA (3)** jenis asas pad berbantuan lakaran mudah.  
(3 markah)
- (b) Jelaskan dengan ringkas **TIGA (3)** kelebihan menggunakan asas cerucuk berbanding asas pad.  
(6 markah)
- (c) Sebuah asas pad bersaiz  $2.5 \text{ m} \times 4 \text{ m} \times 0.7 \text{ m}$  dibebani oleh beban muktamad sebanyak  $1500 \text{ kN}$  melalui sebuah tiang bersaiz  $400 \text{ mm} \times 400 \text{ mm}$  yang terletak di tengah-tengah asas. Akibat daripada beban angin, asas berkenaan menanggung momen sebanyak  $50 \text{ kNm}$ . Data-data rekabentuk bagi asas berkenaan adalah seperti berikut:

Keupayaan galas tanah	=	$200 \text{ kN/m}^2$
Kekuatan ciri konkrit	=	$35 \text{ MPa}$
Penutup konkrit	=	$40 \text{ mm}$
Tetulang tegangan alahan tinggi		

Berdasarkan data-data rekabentuk berkenaan:

- (i) Rekabentukkan semua tetulang. (5 markah)
- (ii) Semak keupayaan ricih asas. (6 markah)
- (d) Sebuah bangunan empat tingkat akan dibina di atas tanah yang mempunyai profil seperti yang ditunjukkan di dalam Rajah S5. Cadangkan SATU (1) penyelesaian bagi masalah asas bangunan tersebut. (5 markah)

**PEPERIKSAAN AKHIR**

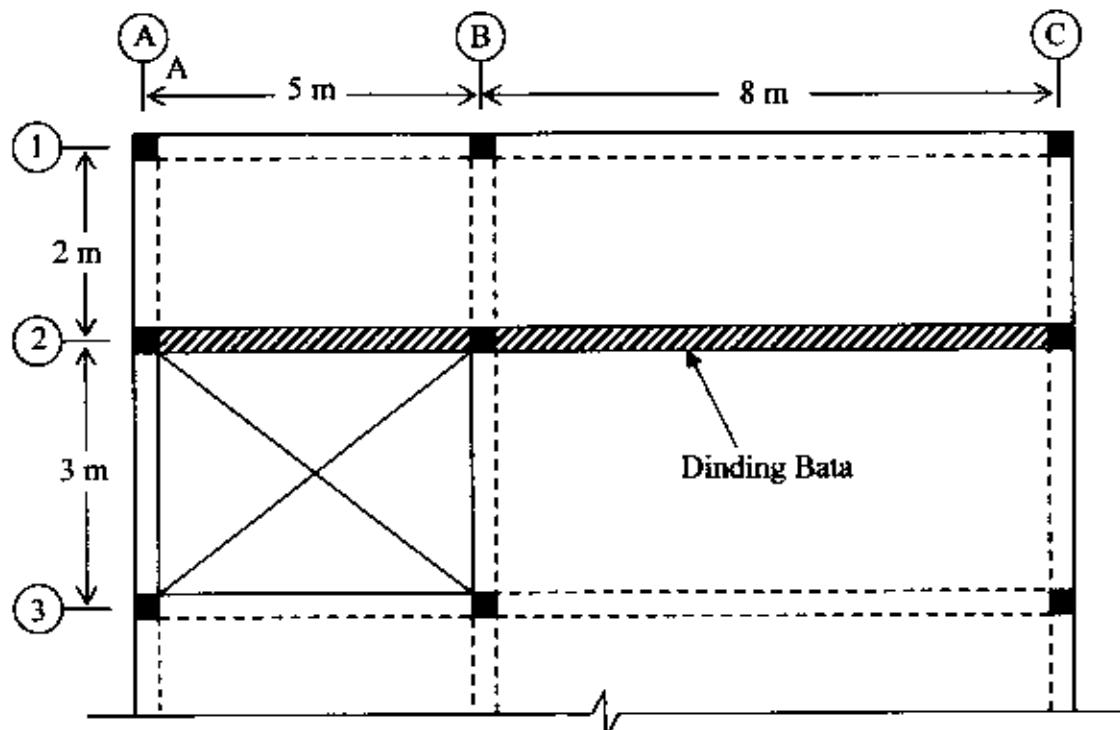
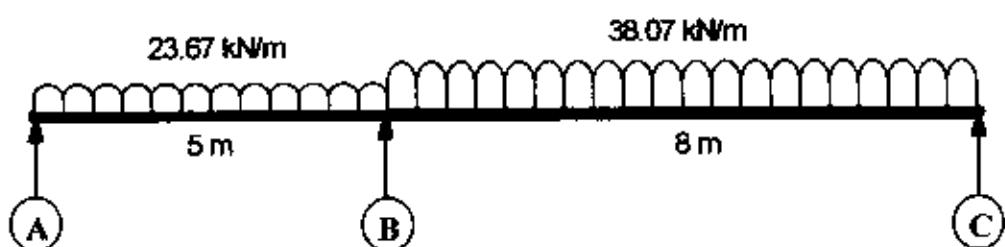
SEMESTER/SESI : SEM II / 2008/2009

KURSUS : BFF / BKA

MATA PELAJARAN : REKABENTUK

KOD MATA PELAJARAN : BFC 3113/BKA 3123

STRUKTUR KONKRIT

**RAJAH S1(a)****RAJAH S1(b)**

**PEPERIKSAAN AKHIR**

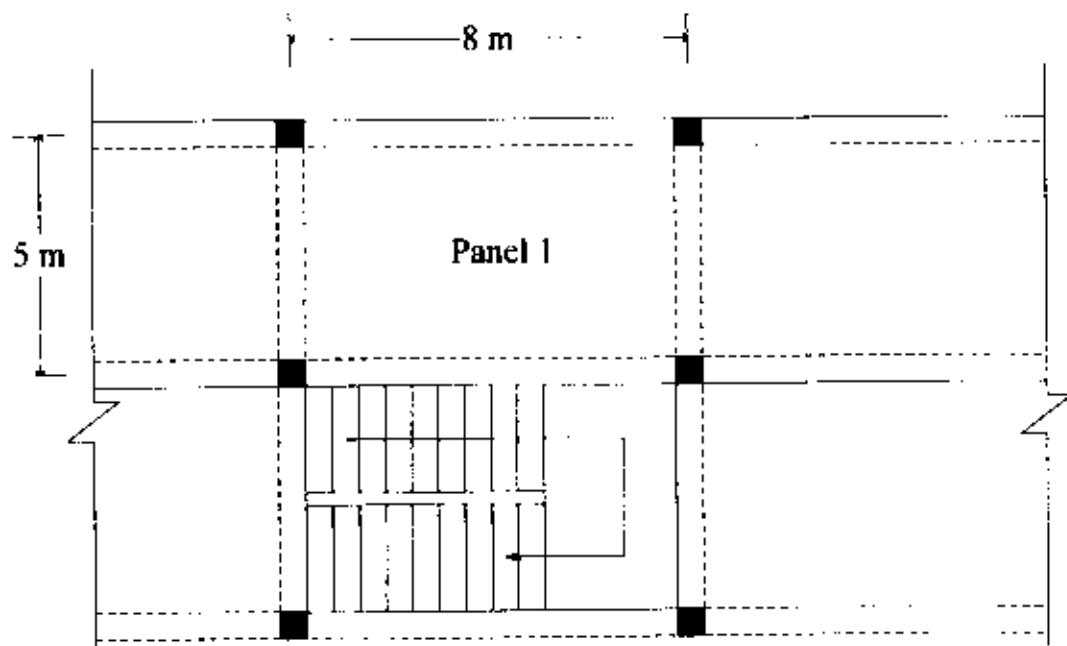
SEMESTER/SESI : SEM II / 2008/2009

KURSUS : BFF / BKA

MATA PELAJARAN :

REKABENTUK  
STRUKTUR KONKRIT

KOD MATA PELAJARAN : BFC 3113/BKA 3123



**RAJAH S2**

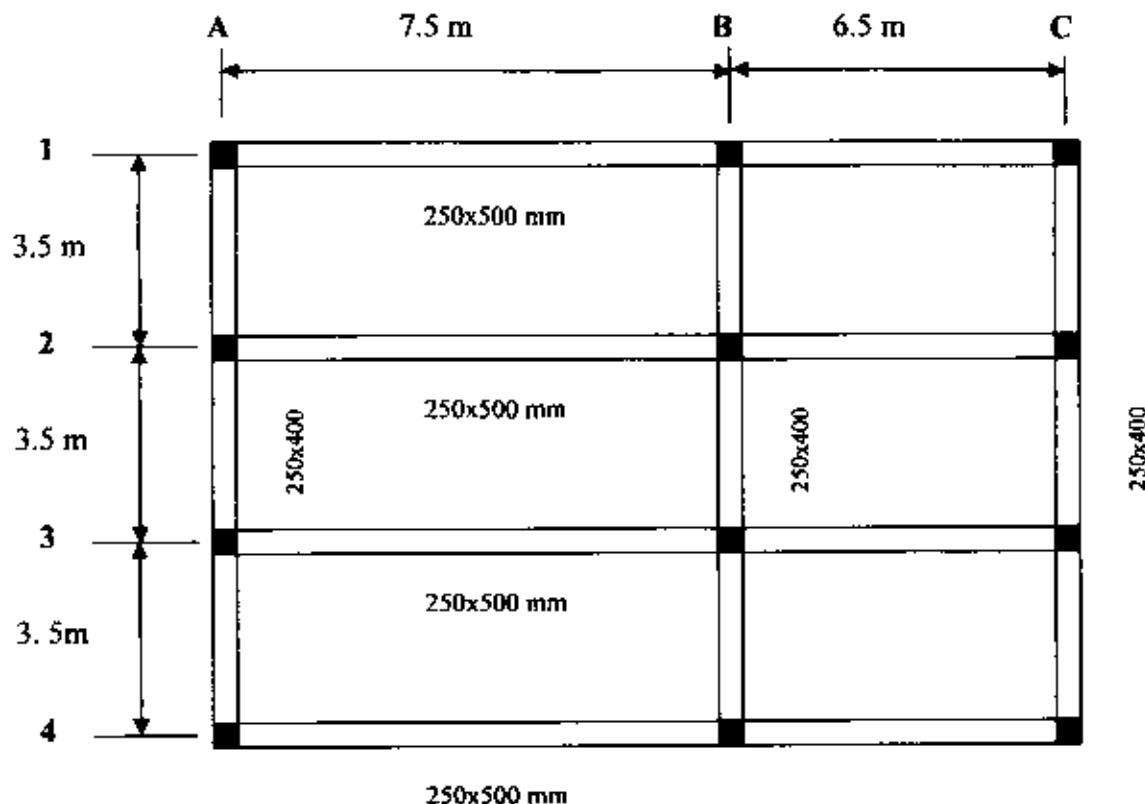
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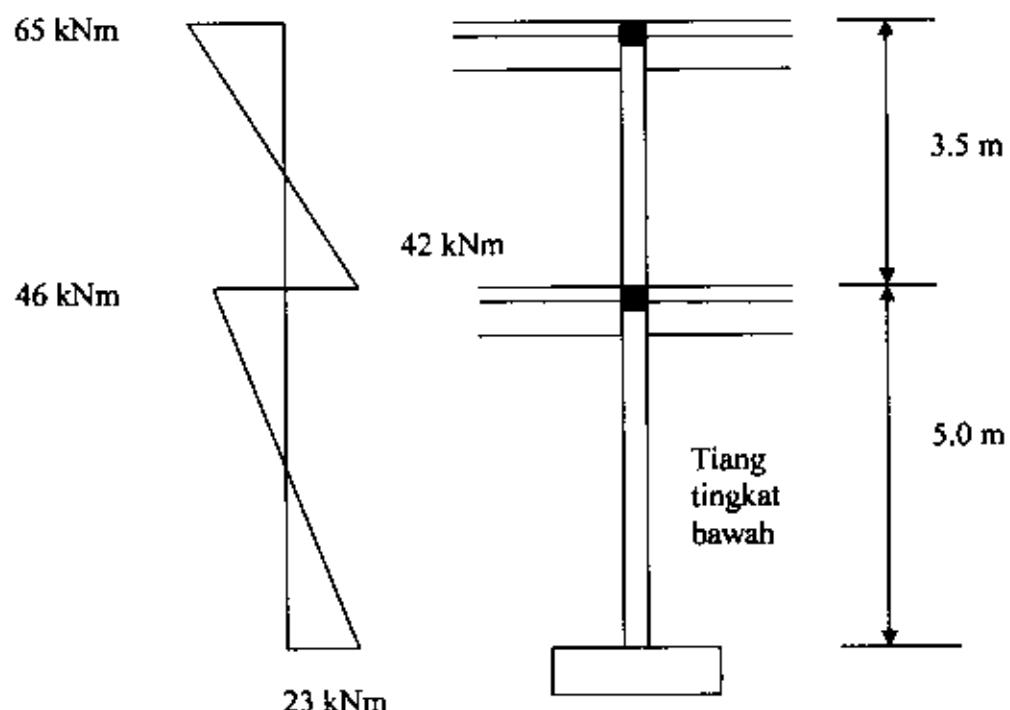
MATA PELAJARAN : REKABENTUK  
STRUKTUR KONKRIT

KURSUS : BFF / BKA

KOD MATA PELAJARAN : BFC 3113/BKA3123



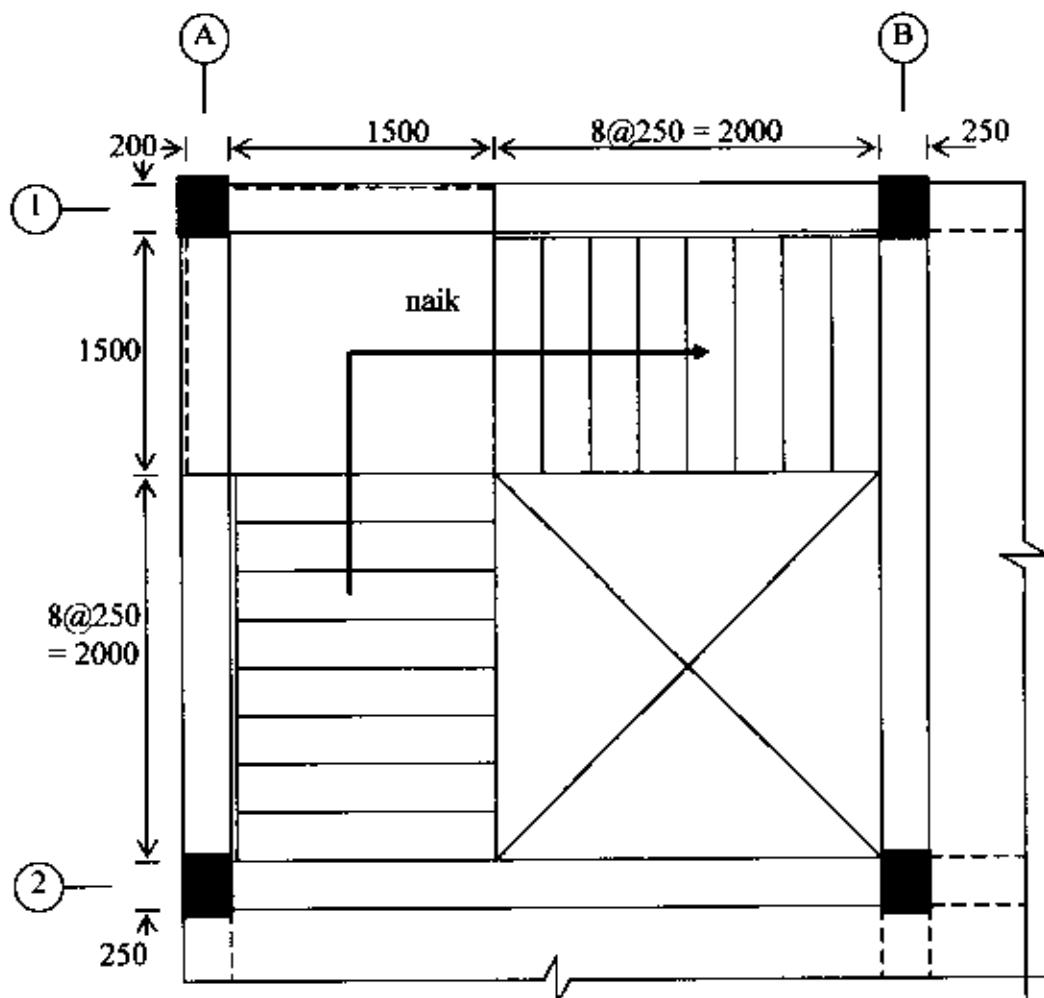
250x500 mm

**RAJAH S3(a)****RAJAH S3(b)****Tiang B/3**

**PEPERIKSAAN AKHIR**

SEMESTER/SESI : SEM II / 2008/2009  
 MATA PELAJARAN : REKABENTUK  
 STRUKTUR KONKRIT

KURSUS : BFF / BKA  
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Semua ukuran dalam mm

**RAJAH S4**

**PEPERIKSAAN AKHIR**

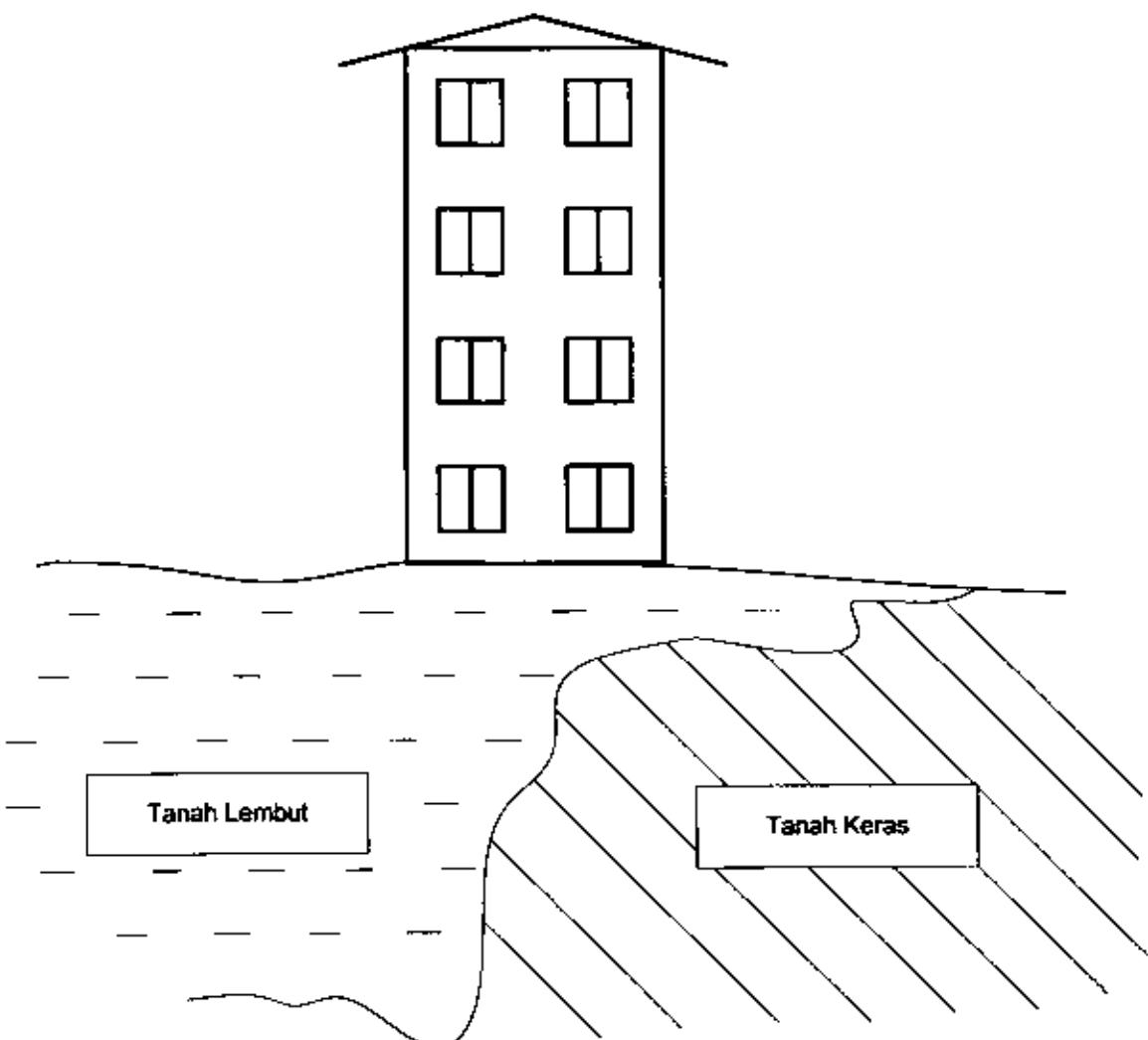
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KURSUS : BFF / BKA

MATA PELAJARAN :

REKABENTUK  
STRUKTUR KONKRIT

KOD MATA PELAJARAN : BFC 3113/BKA3123



**RAJAH S5**

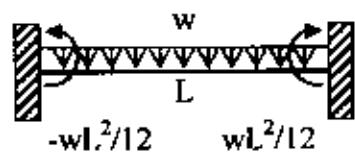
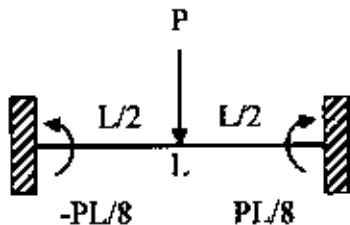
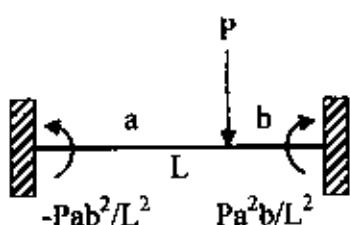
**PEPERIKSAAN AKHIR**

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**Momen Hujung Terikat,  $M^k$ :****Formula:**

$$M_f = (0.45 f_{cu} \cdot b \cdot h_f) \left( d - \frac{h_f}{2} \right)$$

$$Mu_f = 0.156 f_{cu} \cdot b_w \cdot d^2 + (0.45 f_{cu}) (b - b_w) h_f \left( d - \frac{h_f}{2} \right)$$

$$As' = \frac{M - Mu_f}{0.95 f_y (d - d')}$$

$$As = \frac{0.2 f_{cu} \cdot b_w \cdot d + 0.45 f_{cu} (b - b_w) h_f}{0.95 f_y} + As'$$

## PEPERIKSAAN AKHIR

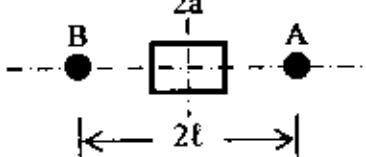
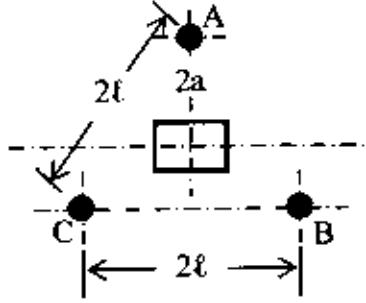
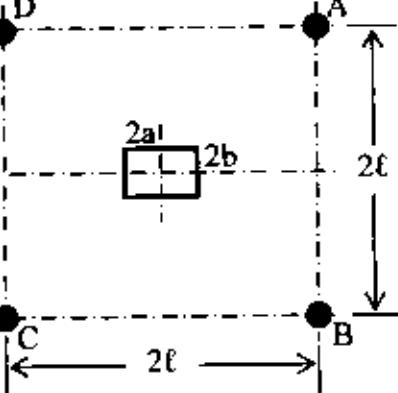
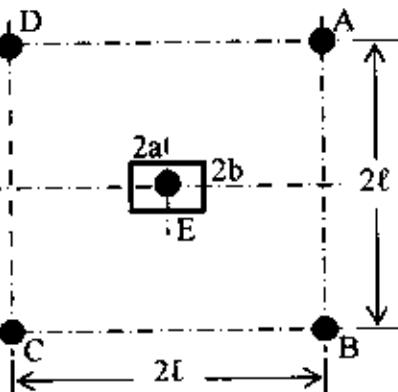
SEMESTER/SESI : SEM II / 2008/2009

KURSUS : BFF / BKA

MATA PELAJARAN : REKABENTUK  
STRUKTUR KONKRIT

KOD MATA PELAJARAN : BFC 3113/BKA3123

## LAMPIRAN A

Bil.	Kumpulan Cerucuk	Daya Tegangan Tukup Cerucuk	
		Saiz tiang diambil kira	Saiz tiang tidak diambil kira
1		$T_{AB} = \frac{N}{6l.d} (3l^2 - a^2)$	$T_{AB} = \frac{Nl}{2.d}$
2		$T_{BC} = \frac{N}{18l.d} (4l^2 + b^2 - 3a^2)$ $T_{AB} = T_{AC} = \frac{N}{9l.d} (2l^2 - b^2)$	$T_{AB} = T_{BC} = T_{CA}$ $= \frac{Nl}{2d}$
3		$T_{AB} = T_{CD} = \frac{N}{12l.d} (3l^2 - b^2)$ $T_{AD} = T_{BC} = \frac{N}{12l.d} (3l^2 - a^2)$	$T_{AB} = T_{BC} = T_{CD} = T_{DA}$ $= \frac{Nl}{4d}$
4		$T_{AB} = T_{CD} = \frac{0.8N}{12l.d} (3l^2 - b^2)$ $T_{AD} = T_{BC} = \frac{0.8N}{12l.d} (3l^2 - a^2)$	$T_{AB} = T_{BC} = T_{CD} = T_{DA}$ $= \frac{0.8Nl}{4d}$

**PEPERIKSAAN AKHIR**

SEMESTER/SESI : SEM II / 2008/2009

KURSUS

: BPP / BKA

MATA PELAJARAN : REKABENTUK  
STRUKTUR KONKRIT

KOD MATA PELAJARAN : BFC 3113/BKA3123

**LAMPIRAN B****Jadual 1: Luas Keratan Rentas Menurut Saiz Serta Bilangan Bar**

Saiz Bar (mm)	Bilangan Bar								Ukur Lilit (mm)
	1	2	3	4	5	6	7	8	
6	28.3	56.6	84.9	113	141	170	198	226	18.9
8	50.3	101	151	201	251	302	352	402	25.1
10	78.6	157	236	314	393	471	550	629	31.4
12	113	226	339	453	566	679	792	905	37.7
16	201	402	603	805	1006	1207	1408	1609	50.3
20	314	629	943	157	1571	1886	2200	2514	62.9
25	491	982	1473	1964	2455	2946	3438	3929	78.6
32	805	1609	2414	3218	4023	4827	5632	6437	100.6
40	1257	2514	3771	5029	6286	7543	8800	10057	125.7

**Jadual 2: Luas Keratan Rentas Bagi Setiap Meter Lebar Untuk Pelbagai Jarak Antara Bar**

Saiz Bar (mm)	Jarak Antara Bar (mm)								
	50	75	100	125	150	175	200	250	300
6	566	377	283	226	189	162	141	113	94
8	1006	670	503	402	335	287	251	201	168
10	1571	1048	786	629	524	44	393	314	262
12	2263	1509	1131	905	754	647	566	453	377
16	4023	2682	2011	1609	1341	1796	1571	1257	1048
20	6286	4190	3143	2514	2095	1796	1571	1257	1048
25	9821	6548	4911	3929	3274	2806	2455	1964	1637
32	16091	10728	8046	6437	5364	4598	4023	3218	2682
40	25143	16762	12571	10057	8381	7184	6286	5029	4190

- Q1** Figure Q1(a) shows a part of ground floor area of double story house which is slab and beam are monolithic. By using the data given:

Slab thickness	=	125 mm
Finishes	=	1.0 kN/m <sup>2</sup>
Brick wall, height : 3 m	=	2.6 kN/m <sup>2</sup>
Concrete Grade	=	30 N/mm <sup>2</sup>
Concrete cover	=	25 mm
Characteristic imposed load	=	2.5 kN/m <sup>2</sup>
Main reinforcement, $\phi_{bar}$ : 20 mm	=	High yield steel
Shear reinforcement, $\phi_{shear}$ : 8 mm	=	Mild steel
Beam size, b <sub>w</sub> x h	=	250 mm x 500 mm

- (a) Prove that the total design load on beam 2/A-C is similar as shown in Figure Q1(b). (4 marks)
- (b) Design beam 2/A-C if the maximum shear force and bending moment from structural analysis are 281.96 kN and 216.93 kNm respectively. (13 marks)
- (c) Check the serviceability limit state of that beam in terms of deflection and cracking. (5 marks)
- (d) Give your comments if the T or L beam was designed as a rectangular beam. (4 marks)

- Q2**
- (a) Define slab. (3 marks)
  - (b) List with aid of sketches THREE (3) types of slab. (3 marks)
  - (c) Figure Q2 shows a continuous slab of 150 mm thickness. Given the following data:

Concrete density	= 24 kN/m <sup>3</sup>
Finishes	= 0.5 kN/m <sup>2</sup>
Characteristics Imposed Load	= 2.5 kN/m <sup>2</sup>
Concrete Grade	= 35 N/mm <sup>2</sup>
Steel Grade	= 460 N/mm <sup>2</sup>
Concrete Cover	= 20 mm
Bar diameter (Assumed)	= 16 mm

For slab Panel 1,

- (i) Design all reinforcement required for Panel 1. Ignore torsional reinforcement. (5 marks)

- (ii) Check for shear. (3 marks)
  - (iii) Check the cracking and deflection. (3 marks)
  - (iv) Draw the detailing. (3 marks)
- (d) A solid slab is supported on its four edges transfer the load through beam. However, in some cases such as flat slab, loading from the slab is transferred directly through the column. Discuss the critical design criteria for the flat slab. (5 marks)

**Q3** Figure Q3(a) shows a plan view of a braced two storey reinforced concrete building. The size of all columns is 250 x 300 mm and the size of the main and secondary beam is 250 x 500 mm and 250 x 400 mm respectively. Figure Q3(b) shows the bending moment values about the X-X axis of column B/3 analysed using sub-frame analysis. The following design data are given:

(i) Loading (all level including the roof)

Main beam:

Characteristic dead load (including self weight)	= 16 kN/m
Characteristic imposed load	= 20 kN/m

Secondary beam:

Characteristic dead load (including self weight)	= 12 kN/m
Characteristic imposed load	= 15 kN/m

(ii) Material properties:

Characteristics strength of concrete	= 35 N/mm <sup>2</sup>
Characteristics strength of main steel	= 460 N/mm <sup>2</sup>
Characteristics strength of link	= 250 N/mm <sup>2</sup>
Concrete cover	= 30 mm

For the ground floor column B/3,

- (a) Classify the type of column. (4 marks)
- (b) Calculate the axial load on the column. (6 marks)
- (c) Design all the reinforcement for the column by assuming the bending moment about Y-Y axis is negligible. (10 marks)

- (d) In your opinion, what design changes has to be made if a circular column is used instead of the rectangular column. (5 marks)

- Q4** Figure Q4 shows a simply supported staircase supported by beam 2/A-B and B/1-2. The landing is supported by beam A/1-2 and 1/1-B. The characteristics imposed load is  $5.0 \text{ kN/m}^2$  and finishing is  $0.75 \text{ kN/m}^2$ . The construction materials consist of grade 30 concrete and high tensile steel reinforcement. The following design data are given:

Going, G	=	250 mm
Rise, R	=	170 mm
Thickness (staircase)	=	250 mm
Thickness (landing)	=	175 mm
Concrete Cover	=	25 mm

- (a) Design all reinforcements for staircase and landing. (11 marks)
- (b) Check for shear, deflection and cracking. (6 marks)
- (c) Sketch the detailing. (4 marks)
- (d) If the deflection checking is not adequate, suggest possible solution to solve the problem. (4 marks)

- Q5** (a) List THREE (3) types foundation with the aided of sketches. (3 marks)
- (b) Briefly explain THREE (3) advantages of pile foundation compared to pad foundation. (6 marks)
- (c) A pad foundation of size of  $2.5 \text{ m} \times 4 \text{ m} \times 0.7 \text{ m}$  is subjected by an ultimate load of  $1500 \text{ kN}$  through a column size  $400 \text{ mm} \times 400 \text{ mm}$  at the center of the footing. Due to wind load, the foundation also encounters moment of  $50 \text{ kNm}$ . The following design data are given:

Soil Bearing Capacity	=	$200 \text{ kN/m}^2$
Characteristics strength of concrete	=	$35 \text{ MPa}$
Concrete Cover	=	$40 \text{ mm}$
High yield steel		

Based on the given data,

- (i) Design all reinforcements. (5 marks)
- (ii) Check shears capacity. (6 marks)
- (d) A four storey residential building will be built on a land with the soil profiles as shown in Figure Q5. Suggest a solution for the foundation problem of the building. (5 marks)

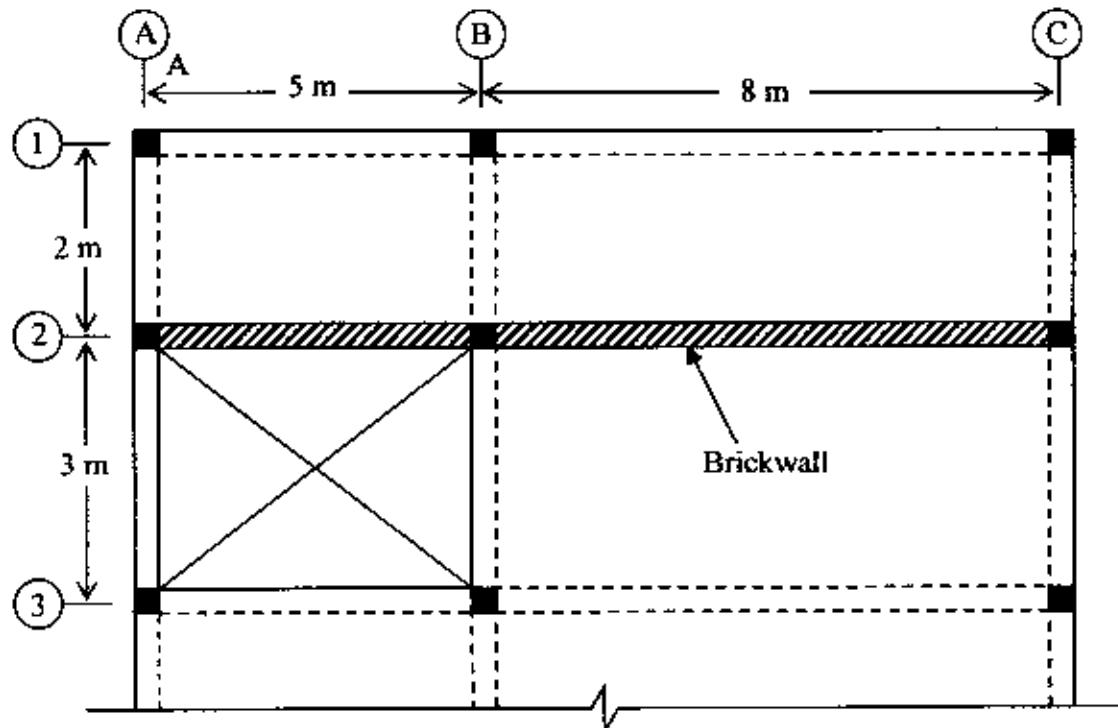
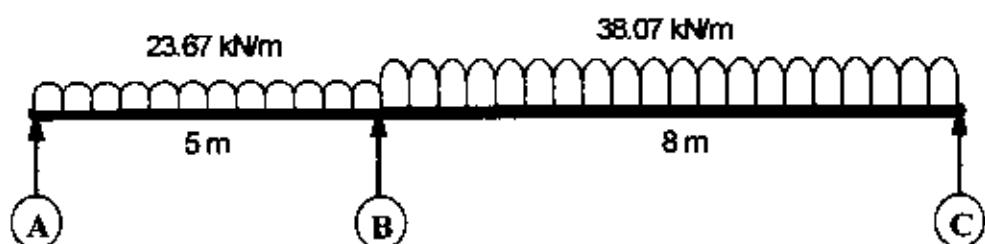
**FINAL EXAMINATION**

SEMESTER/SESSION : SEM II / 2008/2009

SUBJECT : DESIGN OF CONCRETE  
STRUCTURES

COURSE : BFF / BKA

SUBJECT CODE : BFC 3113/BKA 3123

**FIGURE Q1(a)****FIGURE Q1(b)**

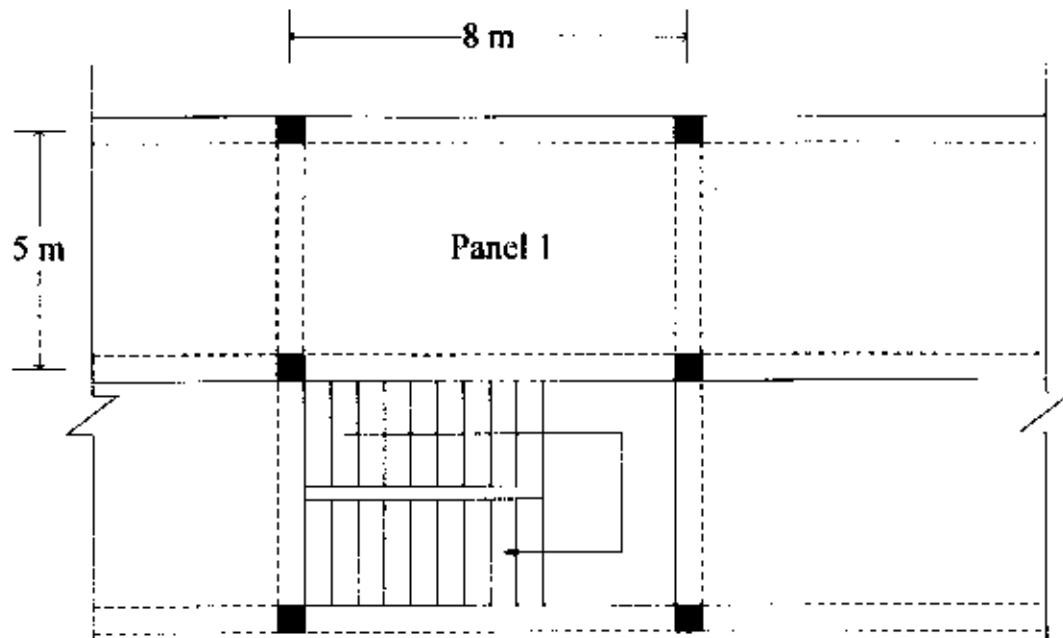
**FINAL EXAMINATION**

SEMESTER/SESSION : SEM II / 2008/2009

COURSE : BFF / BKA

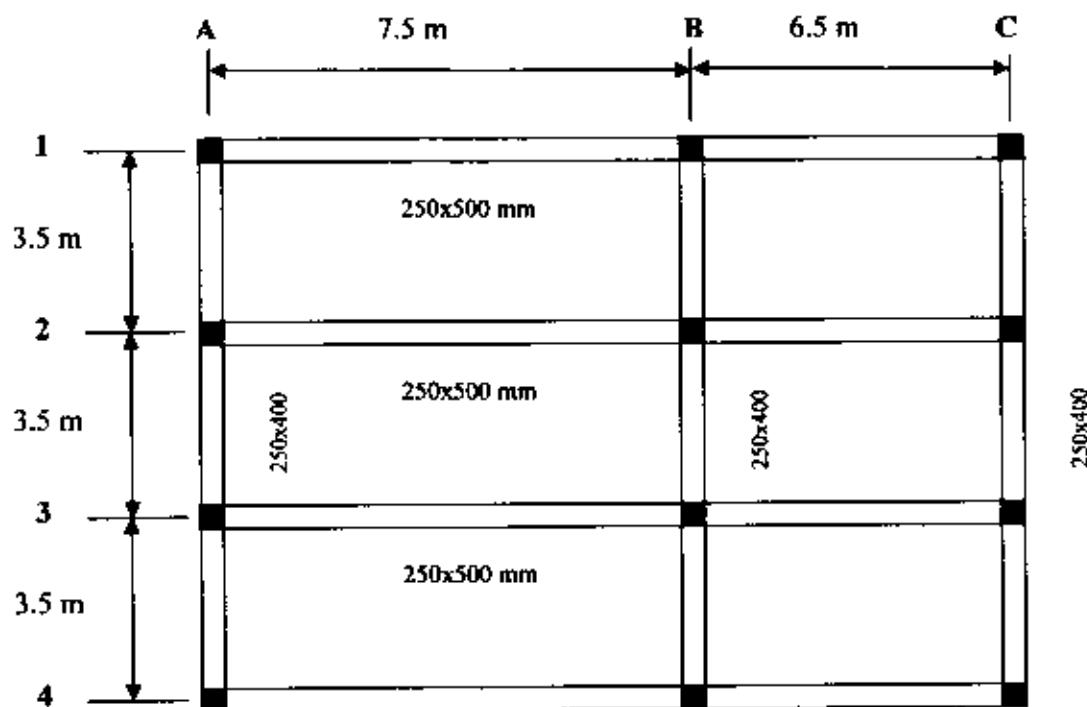
SUBJECT : DESIGN OF CONCRETE  
STRUCTURES

SUBJECT CODE : BFC 3113/BKA 3123

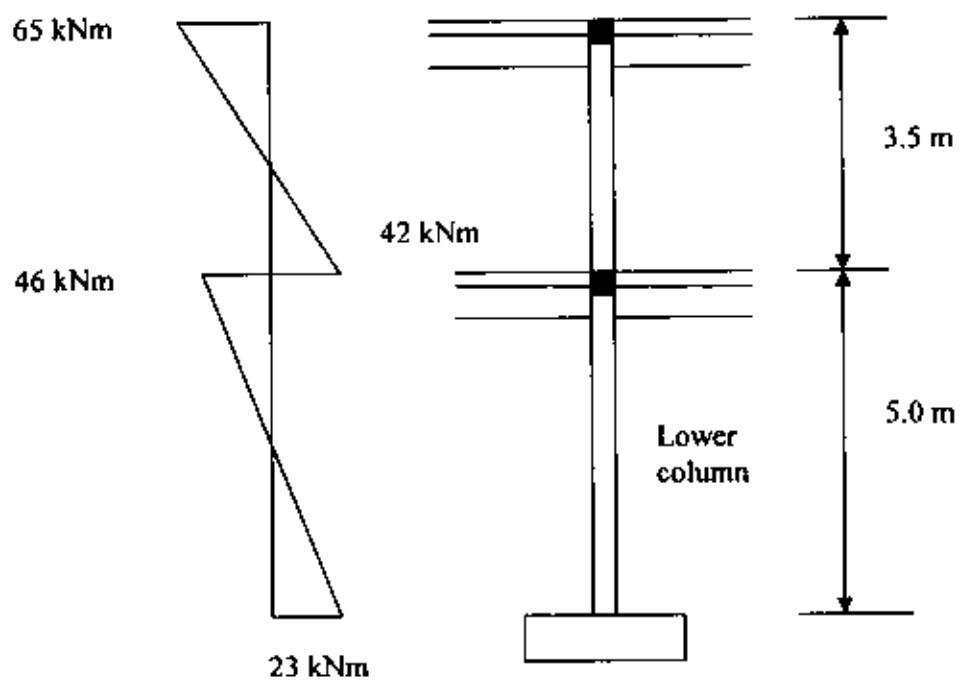
**FIGURE Q2**

**FINAL EXAMINATION**

SEMESTER/SESSION	: SEM II / 2008/2009	COURSE	: BFF / BKA
SUBJECT	DESIGN OF CONCRETE STRUCTURES	SUBJECT CODE	: BFC 3113/BKA 3123



250x500 mm

**FIGURE Q3(a)****FIGURE Q3(b)****Column B/3**

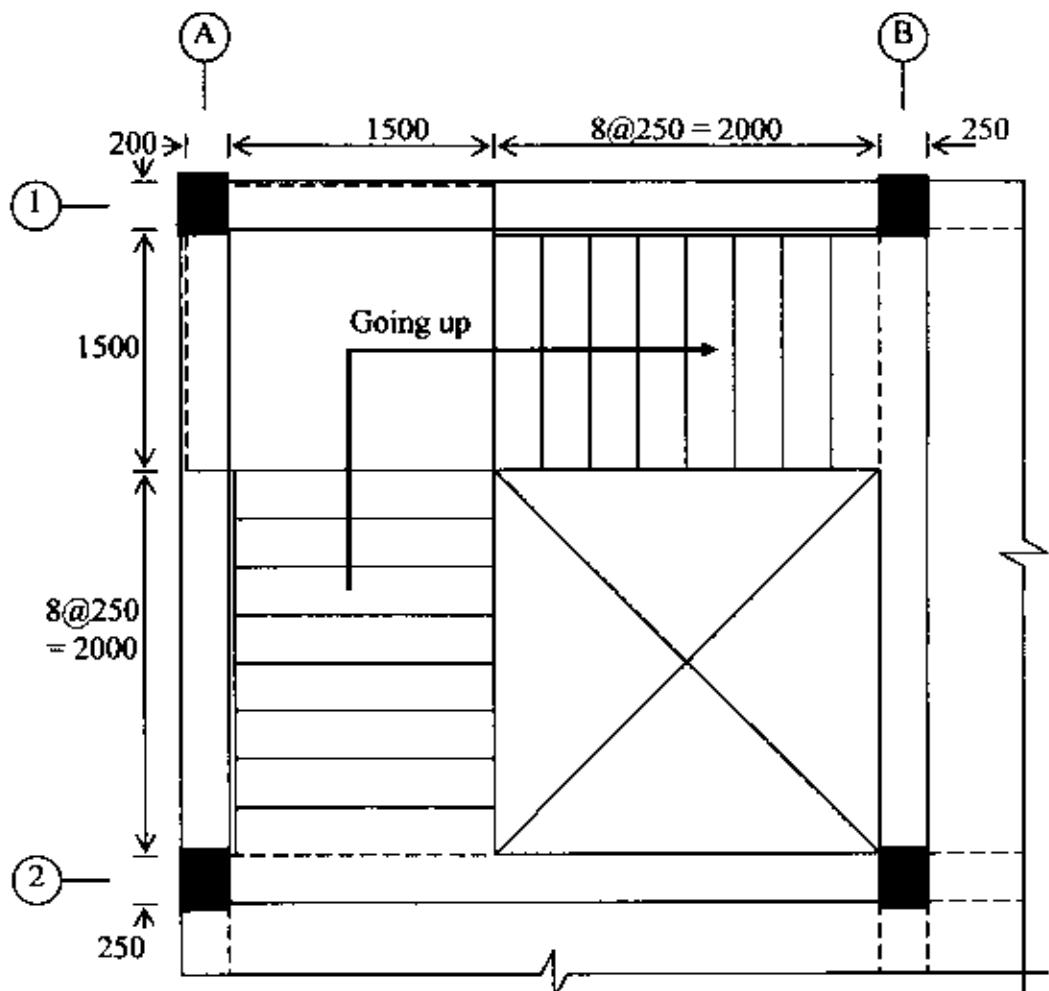
**FINAL EXAMINATION**

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STRUCTURES

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All units in mm

**FIGURE Q4**

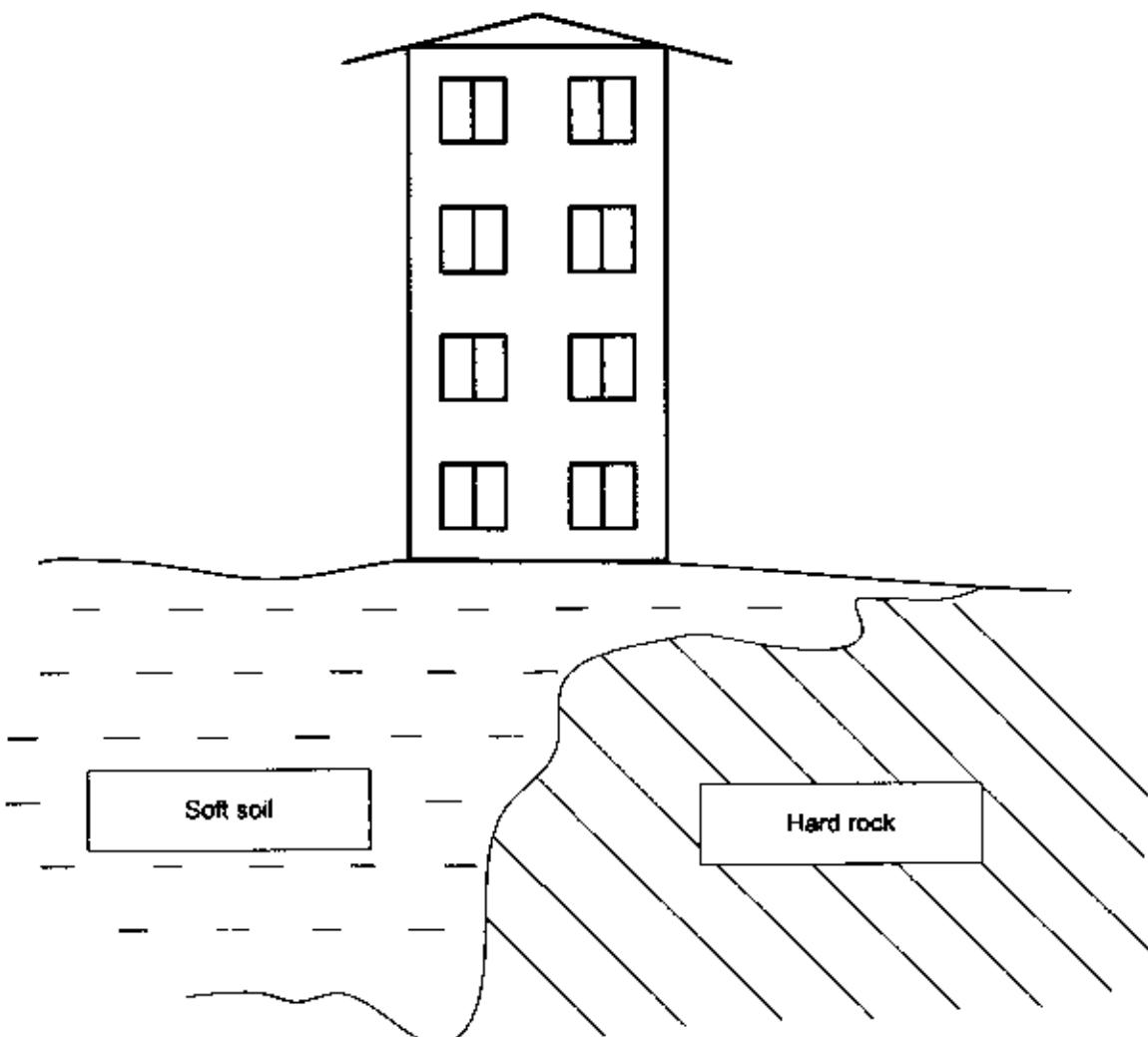
**FINAL EXAMINATION**

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STRUCTURES

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**FIGURE Q5**

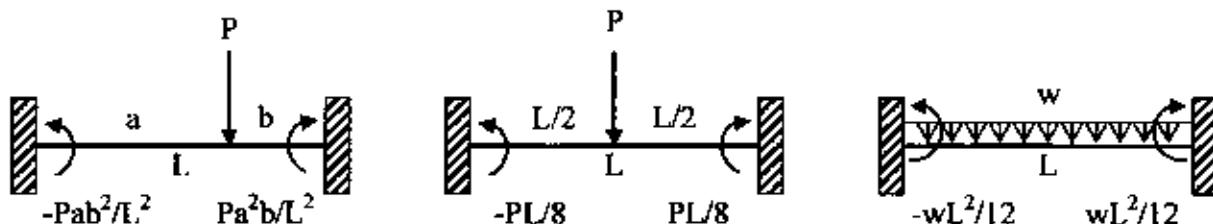
**FINAL EXAMINATION**

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COURSE : BFF / BKA

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STRUCTURES

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***Fix End Moment, FEM:******Formula:***

$$M_f = \left( 0.45 f_{cu} \cdot b \cdot h_f \right) \left( d - \frac{h_f}{2} \right)$$

$$Mu_f = 0.156 f_{cu} \cdot b_w \cdot d^2 + \left( 0.45 f_{cu} \right) \left( b - b_w \right) h_f \left( d - \frac{h_f}{2} \right)$$

$$As' = \frac{M - Mu_f}{0.95f_y(d - d')}$$

$$As = \frac{0.2f_{cu} \cdot b_w \cdot d + 0.45f_{cu}(b - b_w)h_f}{0.95f_y} + As'$$

**FINAL EXAMINATION**

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**APPENDIX A**

No.	Pile Group	Tensile Forces Across Pilecap	
		Accurate	Simplified
1		$T_{AB} = \frac{N}{61d} (3l^2 - a^2)$	$T_{AB} = \frac{Nl}{2d}$
2		$T_{BC} = \frac{N}{181d} (4l^2 + b^2 - 3a^2)$ $T_{AB} = T_{AC} = \frac{N}{91d} (2l^2 - b^2)$	$T_{AB} = T_{BC} = T_{CA} = \frac{Nl}{2d}$
3		$T_{AB} = T_{CD} = \frac{N}{121d} (3l^2 - b^2)$ $T_{AD} = T_{BC} = \frac{N}{121d} (3l^2 - a^2)$	$T_{AB} = T_{BC} = T_{CD} = T_{DA} = \frac{Nl}{4d}$
4		$T_{AB} = T_{CD} = \frac{0.8N}{121d} (3l^2 - b^2)$ $T_{AD} = T_{BC} = \frac{0.8N}{121d} (3l^2 - a^2)$	$T_{AB} = T_{BC} = T_{CD} = T_{DA} = \frac{0.8Nl}{4d}$

**FINAL EXAMINATION**

SEMESTER/SESSION : SEM II / 2008/2009

COURSE : BFF / BKA

SUBJECT : DESIGN OF CONCRETE  
STRUCTURES

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**Appendix B****Table 1: Reinforcement Bars Area**

Bar Size (mm)	Number of Bars								Perimeter (mm)
	1	2	3	4	5	6	7	8	
6	28.3	56.6	84.9	113	141	170	198	226	18.9
8	50.3	101	151	201	251	302	352	402	25.1
10	78.6	157	236	314	393	471	550	629	31.4
12	113	226	339	453	566	679	792	905	37.7
16	201	402	603	805	1006	1207	1408	1609	50.3
20	314	629	943	157	1571	1886	2200	2514	62.9
25	491	982	1473	1964	2455	2946	3438	3929	78.6
32	805	1609	2414	3218	4023	4827	5632	6437	100.6
40	1257	2514	3771	5029	6286	7543	8800	10057	125.7

**Table 2: Reinforcement Bars Area Per Meter Width**

Bar Size (mm)	Bar Distance (mm)								
	50	75	100	125	150	175	200	250	300
6	566	377	283	226	189	162	141	113	94
8	1006	670	503	402	335	287	251	201	168
10	1571	1048	786	629	524	44	393	314	262
12	2263	1509	1131	905	754	647	566	453	377
16	4023	2682	2011	1609	1341	1796	1571	1257	1048
20	6286	4190	3143	2514	2095	1796	1571	1257	1048
25	9821	6548	4911	3929	3274	2806	2455	1964	1637
32	16091	10728	8046	6437	5364	4598	4023	3218	2682
40	25143	16762	12571	10057	8381	7184	6286	5029	4190