

**CONFIDENTIAL**



**UNIVERSITI TUN HUSSEIN ONN MALAYSIA**

**FINAL EXAMINATION  
SEMESTER II  
SESSION 2016/2017**

COURSE NAME : GEOTECHNICS I  
COURSE CODE : BFC 21702  
PROGRAMME CODE : BFF  
EXAMINATION DATE : JUNE 2017  
DURATION : 2 HOURS  
INSTRUCTION : ANSWER ALL QUESTIONS OF  
PART A AND ANY TWO (2)  
QUESTIONS OF PART B

**TERBUKA**

THIS QUESTION PAPER CONSISTS OF ELEVEN (11) PAGES

**CONFIDENTIAL**

## PART A

- Q1** (a) Describe the triaxial compression test for the determination of the shearing resistance of soils, explaining the principles of **THREE (3)** types of test which are commonly conducted. (6 marks)
- (b) Describe the essential features of the direct shear test and discuss typical results obtained from the direct shear test conducted on loose sand. (6 marks)
- (c) The shear strength of soil can be determined in the laboratory and in-situ. The in-situ test has the advantages compared to laboratory testing and it can be performed using various methods.
- (i) List **THREE (3)** advantages and **TWO (2)** disadvantages of shear strength test in the field. (5 marks)
- (ii) List **FIVE (5)** field methods for determining the shear strength in-situ. (5 marks)
- (d) Shear box test result on compacted sand is shown in **Table 1**.
- (i) If the shear box is  $60 \text{ mm}^2$ , estimate the shear strength parameters of the compacted sand. (8 marks)
- (ii) Determine whether the failure occur on a plane within this soil at a point where the normal stress is  $320 \text{ kN/m}^2$  and the corresponding shear stress is  $138 \text{ kN/m}^2$ . (4 marks)
- (iii) If an unconsolidated undrained triaxial test is carried out on a specimen of a similar soil with a cell pressure of  $160 \text{ kN/m}^2$ , find the total axial stress at which failure would be expected. (6 marks)

## PART B

- Q2** (a) Discuss the differences between the Unified Soil Classification System and AASHTO soil classification system (5 marks)
- (b) Mechanical analysis on three different samples denoted as A, B and C were carried out in a soil laboratory. The results of tests are given in **Table 2**. The soil is non plastic.
- (i) Plot the particle size distribution of sample A, B and C on the semi log graph in Figure **Q2(b)(i)**. (6 marks)
- (ii) Determine the effective size, coefficient of uniformity and coefficient of curvature of each sample. (5 marks)
- (iii) Classify the soils according to the Unified Soil Classification System in Figure **Q2(b)(iii)**. (4 marks)
- (c) Earth is required to be excavated from borrow pits for building embankment as shown in Figure **Q2(c)**. The wet unit weight of undisturbed soil of the borrow pits is  $18 \text{ kN/m}^3$  and its water content is 8%, in order to build a 4 m high embankment with top width 2 m and side slope 1:1. Meanwhile, the dry unit weight required in the embankment is  $15 \text{ kN/m}^3$  with a moisture content of 10%.
- (i) Determine the volume of embankment per unit meter length. (1 mark)
- (ii) Calculate the the dry unit weight of soil in the borrow pit. (2 marks)
- (iii) Determine the volume of earth required to be excavated,  $V$  per meter from borrow pit for embankment with dry unit weight  $15 \text{ kN/m}^3$ . (2 marks)
- (iv) Calculate the void ratios and the degree of saturation of embankment at undisturbed state. (5 marks)

- Q3** (a) Explain briefly with a suitable table, the standard proctor compaction test and modified proctor compaction test. (6 marks)
- (b) The results of compaction test in the laboratory using the standard proctor method for the Batu Pahat residual soil are shown in Figure **Q3(b)**. After compaction of the soil in the laboratory, the field density tests using the sand cone replacement method were performed. The volume of soil excavated was  $1165 \text{ cm}^3$  and the bulk weight and dry weight of the excavated soil are 2230 g and 1852 g respectively. According to JKR specifications, the relative compaction must be at least 95% and the moisture content should be within  $\pm 2\%$  of optimum moisture content ( $w_{\text{opt}}$ ). Assume the specific gravity,  $G_s = 2.65$ .
- (i) Plot the 0%, 5% and 10% air void lines in Figure **Q3(b)** and determine the maximum dry density and optimum moisture content from compaction curve. (8 marks)
- (ii) Determine the relative compaction of the compacted soil. (3 marks)
- (iii) Does the compaction meet the JKR specification? Please explain. (4 marks)
- (iv) If the compacted layer in the field was 3 m, calculate the vertical stress ( $\text{kN/m}^3$ ) at a depth of 2 m from the ground surface. (3 marks)
- (c) Figure **Q3(c)** shows a graph of compaction curves A, B and C for the same soil with varying compactive effort. Discuss the most efficient compaction condition to achieve  $R\gamma_{d(\text{max})}$ . Your discussion should include the range of water content and the selected compaction curves. (6 marks)

- Q4** (a) Differentiate between falling head test and constant head test. (4 marks)
- (b) The soil layers below have a cross section of 100 mm x 100 mm each as shown in Figure **Q4(b)**. The permeability of each soil is:  $k_A = 10^{-2}$  cm/sec.;  $k_B = 3 \times 10^{-3}$  cm/sec;  $k_C = 4.9 \times 10^{-4}$  cm/sec. Find the rate of water supply in cm<sup>3</sup>/hr. (9 marks)
- (c) Consider the soil profile shown in Figure **Q4(c)**:
- (i) Calculate the variations of  $\sigma$ ,  $u$ , and  $\sigma'$  at points *A*, *B*, and *C*. (11 marks)
- (ii) How high should the groundwater table rise so that the effective stress at *C* is 111 kN/m<sup>2</sup>? (6 marks)

- END OF QUESTIONS -

**FINAL EXAMINATION**

SEMESTER / SESSION : SEM II /2016/2017      PROGRAMME CODE: BFF  
 COURSE NAME : GEOTECHNICS I      COURSE CODE : BFC 21702

**TABLE 1: Results of Shear Box Test**

Test No.	Normal load (N)	Shear load (N)
1	360	260
2	720	380
3	1080	520
4	1440	640

**TABLE 2: Results of sieve analysis of sample A, B and C**

Samples ASTM Sieve Designation	Percentage Passing (%)		
	A	B	C
63.0 mm	100		93
20.0 mm	64		76
6.3 mm	39	100	65
2.0 mm	24	98	59
600 µm	12	90	54
212 µm	5	9	47
63 µm	1	2	34
20 µm			23
6 µm			7
2 µm			4

**TERBUKA**

FINAL EXAMINATION

SEMESTER / SESSION : SEM II /2016/2017  
COURSE NAME : GEOTECHNICS I

PROGRAMME CODE: BFF  
COURSE CODE : BFC 21702

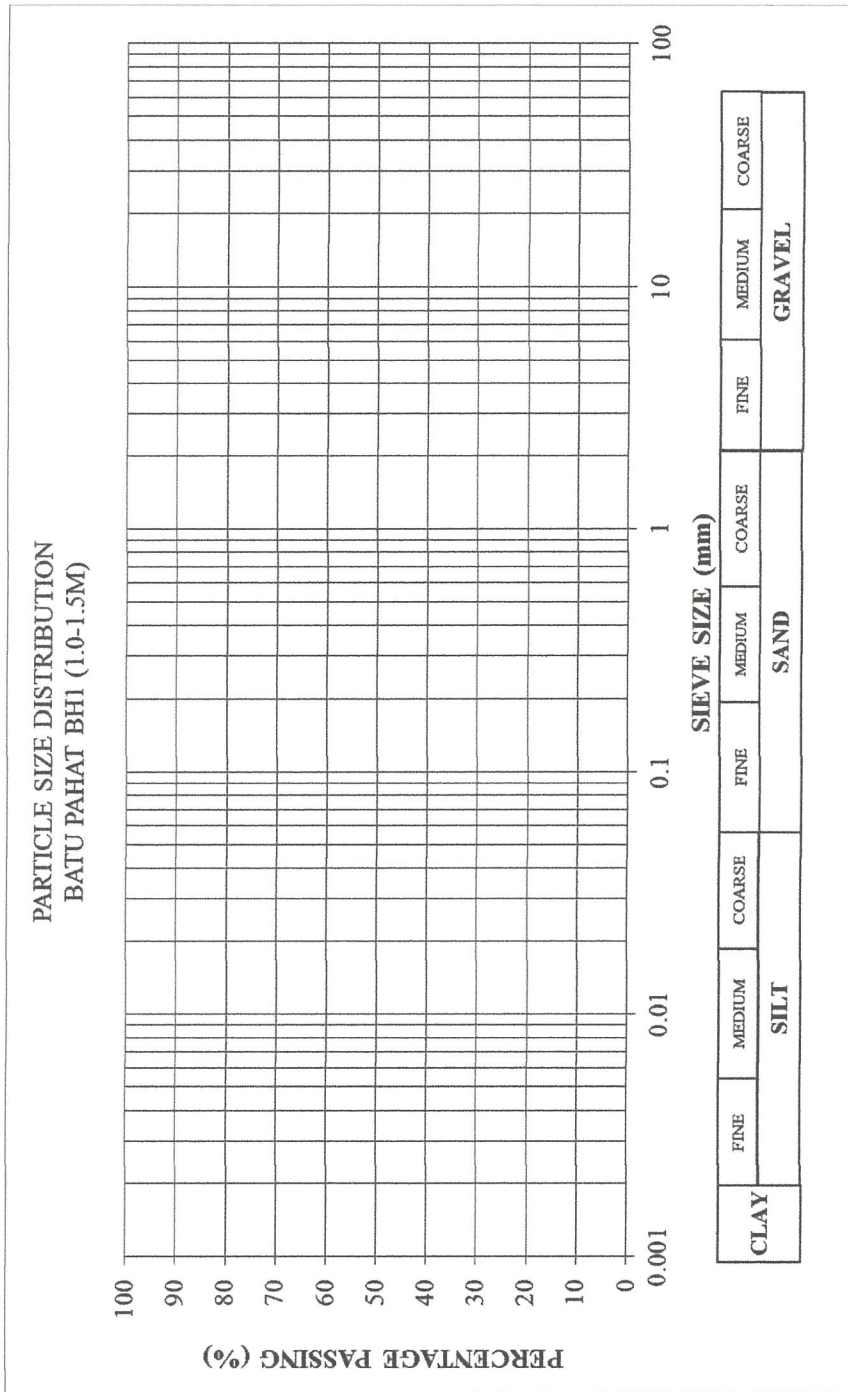


FIGURE Q2 (b)(i) : Particle size distribution chart according to British Standard

TERBUKA

FINAL EXAMINATION

SEMESTER / SESSION : SEM II /2016/2017  
 COURSE NAME : GEOTECHNICS I

PROGRAMME CODE: BFF  
 COURSE CODE : BFC 21702

Criteria for Assigning Group Symbols				Group Symbol
Coarse-Grained Soils More than 50% of retained on No. 200 sieve	Gravels More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels	$C_u \geq 4$ and $1 \leq C_c \leq 3^c$	GW
		Less than 5% fines <sup>a</sup>	$C_u < 4$ and/or $1 > C_c > 3^c$	GP
		Gravels with Fines More than 12% fines <sup>a,d</sup>	$PI < 4$ or plots below "A" line (Figure 4.2) $PI > 7$ and plots on or above "A" line (Figure 4.2)	GM GC
	Sands 50% or more of coarse fraction passes No. 4 sieve	Clean Sands	$C_u \geq 6$ and $1 \leq C_c \leq 3^c$	SW
		Less than 5% fines <sup>b</sup>	$C_u < 6$ and/or $1 > C_c > 3^c$	SP
		Sands with Fines More than 12% fines <sup>b,d</sup>	$PI < 4$ or plots below "A" line (Figure 4.2) $PI > 7$ and plots on or above "A" line (Figure 4.2)	SM SC
Fine-Grained Soils 50% or more passes No. 200 sieve	Silts and Clays Liquid limit less than 50	Inorganic	$PI > 7$ and plots on or above "A" line (Figure 4.2) <sup>e</sup> $PI < 4$ or plots below "A" line (Figure 4.2) <sup>f</sup>	CL ML
		Organic	Liquid limit-oven dried Liquid limit-not dried $< 0.75$ ; see Figure 4.2; OL zone	OL
	Silts and Clays Liquid limit 50 or more	Inorganic	$PI$ plots on or above "A" line (Figure 4.2) $PI$ plots below "A" line (Figure 4.2)	CH MH
		Organic	Liquid limit-oven dried Liquid limit-not dried $< 0.75$ ; see Figure 4.2; OH zone	OH
	Highly Organic Soils	Primarily organic matter, dark in color, and organic odor		Pt

<sup>a</sup>Gravels with 5 to 12% fine require dual symbols: GW-GM, GW-GC, GP-GM, GP-GC.

<sup>b</sup>Sands with 5 to 12% fines require dual symbols: SW-SM, SW-SC, SP-SM, SP-SC.

$$C_u = \frac{D_{60}}{D_{10}}; C_c = \frac{(D_{30})^2}{D_{60} \times D_{10}}$$

<sup>d</sup>If  $4 \leq PI \leq 7$  and plots in the hatched area in Figure 4.2, use dual symbol GC-GM or SC-SM.

<sup>e</sup>If  $4 \leq PI \leq 7$  and plots in the hatched area in Figure 4.2, use dual symbol CL-ML.

USCS	Boulders	Cobbles	Gravel		Sand			Fines (Silt, Clay)
			Coarse	Fine	Coarse	Medium	Fine	
	300	75	19	4.75	2.0	0.425	0.075	

FIGURE Q2 (b)(iii) : USCS Classification





FINAL EXAMINATION

SEMESTER / SESSION : SEM II / 2016/2017  
COURSE NAME : GEOTECHNICS I

PROGRAMME CODE: BFF  
COURSE CODE : BFC 21702

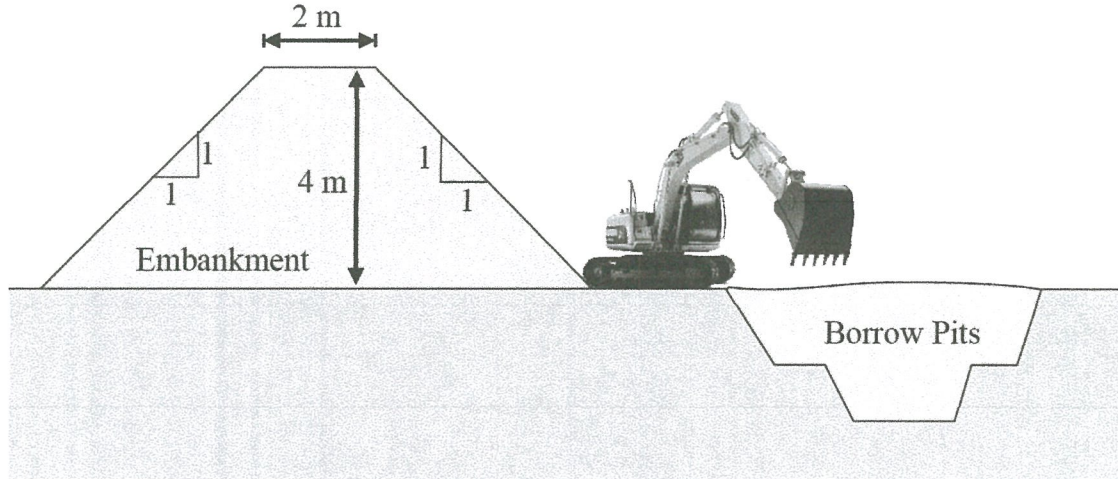


FIGURE Q2(c): Embankment structure and borrow pits

Given that:

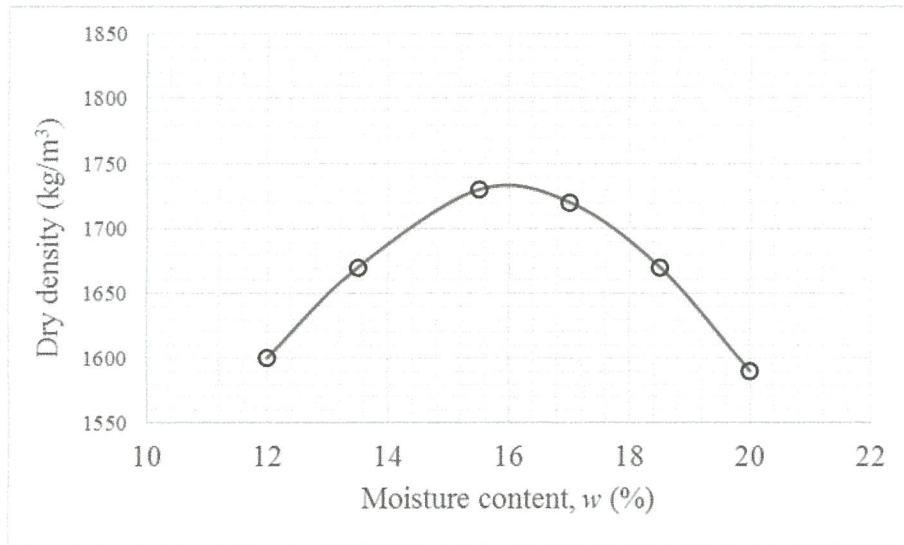


FIGURE Q3(b): Proctor test result

TERBUKA

FINAL EXAMINATION

SEMESTER / SESSION : SEM II /2016/2017  
COURSE NAME : GEOTECHNICS I

PROGRAMME CODE: BFF  
COURSE CODE : BFC 21702

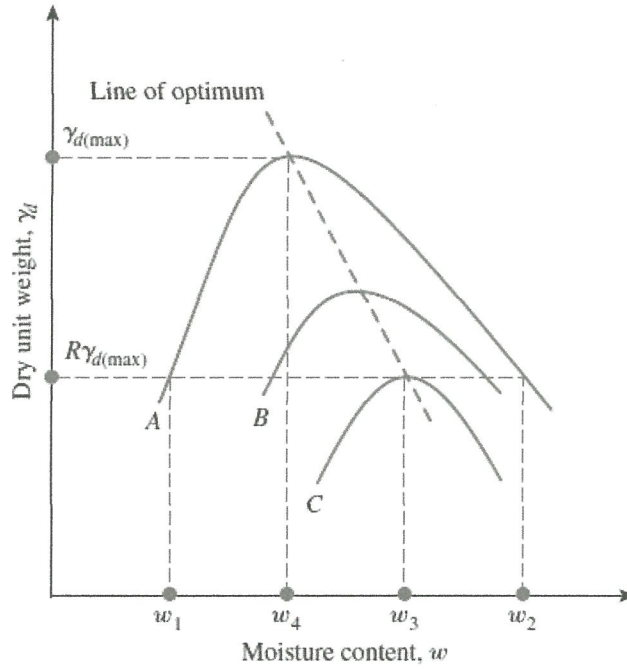


FIGURE Q3(c): Proctor test result for the same soil with varying compactive effort

TERBUKA

FINAL EXAMINATION

SEMESTER / SESSION : SEM II /2016/2017  
 COURSE NAME : GEOTECHNICS I

PROGRAMME CODE: BFF  
 COURSE CODE : BFC 21702

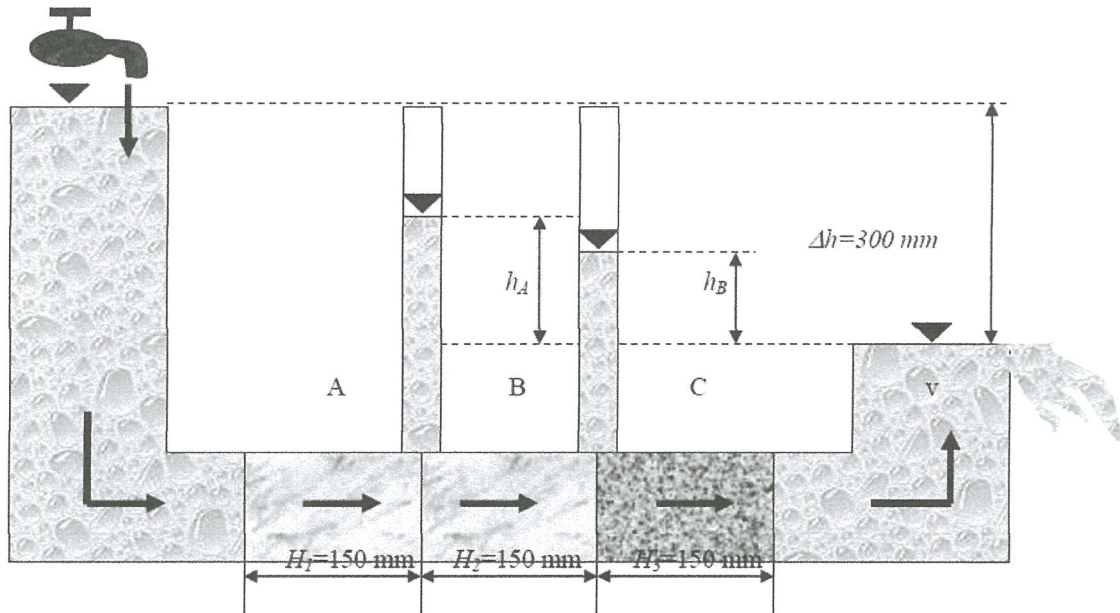


FIGURE Q4(b): Soil layer with different permeability

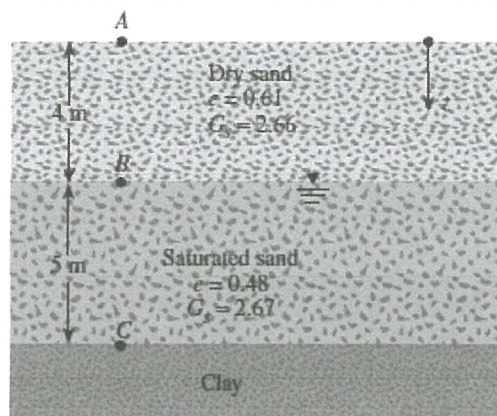


FIGURE Q4(c): Soil profile