



UTHM
Universiti Tun Hussein Onn Malaysia

UNIVERSITI TUN HUSSEIN ONN MALAYSIA

**FINAL EXAMINATION
SEMESTER II
SESSION 2016/2017**

COURSE NAME : HYDROLOGY
COURSE CODE : BFC32002
PROGRAMME CODE : BFF
EXAMINATION DATE : JUNE 2017
DURATION : 2 HOURS 30 MINUTES
INSTRUCTION : ANSWER ONE (1) QUESTION IN SECTION A, AND THREE (3) QUESTIONS IN SECTION B

THIS QUESTION PAPER CONSISTS OF NINE (9) PAGES

PART A

- Q1** (a) Give **THREE (3)** important ecological functions of groundwater and briefly describe each function. (6 marks)
- (b) With the aid of a sketch, explain the differences between confined and unconfined aquifer. (6 marks)
- (c) A 30 cm well fully penetrates an unconfined aquifer of saturated depth 25 cm. When a discharge of 2100 litre per minute was being pumped for a long time, observation wells at radial distances of 30 m and 90 m indicated drawdown of 5 m and 4 m, respectively.
- (i) Estimate the coefficient of permeability and transmissivity of the aquifer. (8 marks)
- (ii) Calculate the drawdown at the pumping well. (5 marks)

PART B

- Q2** (a) State **TWO (2)** types of rainfall. (2 marks)
- (b) An open water tank is set up to collect rainfall for watering a small garden. The data of rainfall, evaporation, and water level are taken daily for a week as tabulated in **Table 1**.
- (i) Calculate water usage (outflow) for 7 days in mm. (7 marks)
- (ii) If there is no rain for the next 3 days consecutively, is the storage enough to water to the plants? (5 marks)
- (c) Analyze a missing station *C* record of precipitation as shown in **Table 2** by quadrant application for this area. (11 marks)

- Q3**
- (a) Define the following terms :
- (i) Infiltration rate (2 marks)
 - (ii) Infiltration capacity (2 marks)
- (b) Explain in detail **ONE (1)** type of Infiltrimeter. (5 marks)
- (c) A basin area of 250 hectare produced a mass curve of the average rainfall depth and was recorded as in the **Table 3**. If the ϕ index was found to be 7.85 mm/hr, determine the runoff volume of the catchment in cubic meter. (8 marks)
- (d) A 24 hours storm occurred over a catchment of 2.5 km² area and the total rainfall observed was 10 cm. An infiltration capacity curve prepared had the initial infiltration capacity of 1 cm/hour and attained a constant value of 0.3 cm/hour with a Horton's constant $k = 5\text{hr}^{-1}$. A pan installed in the catchment area indicated a decrease of 0.6 cm in the water level during the 24 hours of its operation. Calculate the total infiltration loss and volume of runoff (in m³) from the catchment. Assume a pan coefficient of 0.7. (8 marks)
- Q4**
- (a) Define the surface runoff (2 marks)
- (b) Define the current meter and differentiate between vertical-axis and horizontal-axis meter. (6 marks)
- (c) A 150 g/L solution of salt was discharged into a stream at a constant rate of 20 L/s. The background concentration of the salt in the stream was found to be 9 ppm (part per million). At a downstream section, the solution was completely mixed and the salt concentration was found to reach an equilibrium value of 45 ppm. Calculate the discharge in the stream. (3 marks)
- (d) The following data in **Table 4** are obtained from the current meter gauging ($V = 0.23N_s + 0.04$) of a stream. Compute the stream discharge by using the mid section method. (14 marks)

- Q5** (a) Define the followings :
- (i) S-curve. (2 marks)
 - (ii) Time of concentration, t_c . (2 marks)
- (b) Describe the characteristics of a typical hydrograph. (5 marks)
- (c) The streamflow data of a river for a catchment area of 650 km^2 is given in **Table 5**. Plot a hydrograph and separate the baseflow from the direct runoff by using simple method and intersection method. Compare the quantity of the direct runoff of both methods. (Apply both methods on a single graph paper). (8 marks)
- (d) Derive the hydrograph for an excess rainfall of 5.5 mm, 8.5 mm which occurs in the first and second hours. The unit hydrograph ordinates are shown in the **Table 6**. (8 marks)
- Q6** (a) Describe **TWO (2)** usages of reservoir routing method. (4 marks)
- (b) The incremental time period of inflow and outflow of a reservoir is 0.40 hours as shown in **Tables 7 and 8**. Evaluate the design of outflow of $2 \text{ m}^3/\text{s}$. (21 marks)

– END OF QUESTIONS –

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TABLE 1

| Day | Evaporation (mm) | Rainfall (mm) | Water Level in Tank (mm) |
|-----|------------------|---------------|--------------------------|
| 1 | 1.7 | 0 | 1000 |
| 2 | 0 | 35.4 | |
| 3 | 1.7 | 0 | |
| 4 | 0 | 50.8 | |
| 5 | 1.7 | 0 | |
| 6 | 0 | 121.6 | |
| 7 | 1.7 | 0 | 480 |

TABLE 2

| Station | Rainfall Depth (mm) | Coordinate | |
|---------|---------------------|------------|----|
| | | X | Y |
| A | 29 | -5 | -5 |
| B | 21 | -1 | 2 |
| D | 17 | 6 | 1 |
| E | 31 | 1 | 1 |
| F | 28 | 3 | -2 |
| G | 22 | 4 | 5 |
| H | 19 | -5 | 1 |

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TABLE 3

| | | | | | | | |
|--|---|------|------|------|------|-------|-------|
| Time (hour) | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| Accumulated Average Rainfall (mm) | 7 | 15.2 | 60.4 | 81.5 | 97.6 | 120.0 | 120.0 |

TABLE 4

| Distance from one end of the river (m) | Depth of water, d (m) | Current meter reading at 0.6 of depth | |
|--|-----------------------|---------------------------------------|------------|
| | | revolution | time (sec) |
| 0 | 0 | 0 | 0 |
| 1 | 1.1 | 39 | 100 |
| 3 | 2.0 | 58 | 100 |
| 5 | 2.5 | 112 | 150 |
| 7 | 2.0 | 89 | 100 |
| 9 | 1.7 | 45 | 100 |
| 11 | 1.0 | 30 | 100 |
| 12 | 0 | 0 | 0 |

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TABLE 5

| Time (days) | Flow (m ³ /s) |
|-------------|--------------------------|
| 1 | 600 |
| 2 | 1400 |
| 3 | 5000 |
| 4 | 10500 |
| 5 | 7000 |
| 6 | 4200 |
| 7 | 3800 |
| 8 | 3000 |

TABLE 6

| Time (hours) | Excess Rainfall (mm) | Direct Discharge (m ³ /s mm) |
|--------------|----------------------|---|
| 1 | 5.5 | 1 |
| 2 | 8.5 | 3 |
| 3 | | 7 |
| 4 | | 23 |
| 5 | | 86 |
| 6 | | 66 |
| 7 | | 30 |
| 8 | | 10 |
| 9 | | 3 |
| 10 | | 1 |

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TABLE 7

| | | | | | | | |
|---------------------------------|------|------|------|------|------|------|------|
| Time (hour) | 0 | 0.4 | 0.8 | 1.2 | 1.6 | 2.0 | 2.4 |
| Inflow (m³/s) | 0.11 | 0.17 | 0.25 | 0.65 | 2.38 | 1.36 | 0.57 |

| | | | | | | |
|---------------------------------|------|------|------|------|------|------|
| Time (hour) | 2.8 | 3.2 | 3.6 | 4 | 4.4 | 4.8 |
| Inflow (m³/s) | 0.40 | 0.31 | 0.25 | 0.23 | 0.20 | 0.20 |

TABLE 8

| | | | | | |
|--|---|------|------|------|------|
| Outflow (m³/s) | 0 | 0.07 | 0.71 | 1.91 | 3.45 |
| Storage (m³) | 0 | 212 | 1019 | 1698 | 2547 |
| $\frac{2S}{\Delta t} - O$ (m ³ /s) | 0 | 0.22 | 0.71 | 0.45 | 0.09 |
| $\frac{2S}{\Delta t} + O$ (m ³ /s) | 0 | 0.37 | 2.13 | 4.27 | 6.99 |

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The following information may be useful. The symbols have their usual meaning.

$$1 \text{ ha} = 10000 \text{ m}^2$$

$$E = C(e_o - e_a) \left[1 + \frac{W}{10} \right]$$

$$\text{index, } \phi = \frac{P - R}{t_e}$$

$$f = f_c + (f_o - f_c)e^{-kt}$$

$$I - O = \frac{\Delta S}{\Delta t}$$

$$H^2 - h^2 = \frac{Q}{\pi K} \ln \left(\frac{R}{r} \right)$$

$$H - h = \frac{Q}{2\pi bK} \ln \left(\frac{R}{r} \right)$$

$$Q_2 = C_0 I_2 + C_1 I_1 + C_2 I_1$$

$$C_0 = \frac{0.5\Delta t - Kx}{K(1-x) + 0.5\Delta t}$$

$$C_1 = \frac{0.5\Delta t + Kx}{K(1-x) + 0.5\Delta t}$$

$$C_2 = \frac{K(1-x) - Kx}{K(1-x) + 0.5\Delta t}$$

$$P = \sum \left(\frac{P_i}{n} \right)$$

$$T = KB$$

$$U = K_s B$$

$$P_x = \sum (W_i P_i)$$

$$B = \sum \left(\frac{tp}{100} \right)$$

$$(I_1 + I_2) + \left(\frac{2S_1}{\Delta t} - O_1 \right) = \left(\frac{2S_2}{\Delta t} + O_2 \right)$$

$$u = \frac{ktp}{100}$$

$$Q = \frac{C_1 - C_2}{C_2 - C_0} \times q$$